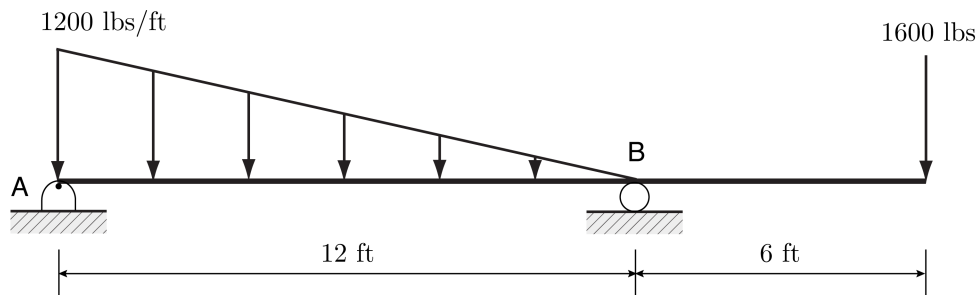


CE 325 HW#2

1. For the determinate beam given below:

- a. (5 pts) Draw shear (V) and bending moment (M) diagrams and label with values at each transition point (beginning/end segments, maxima/minima, points of inflection). Show your sign conventions for the V and M diagrams.

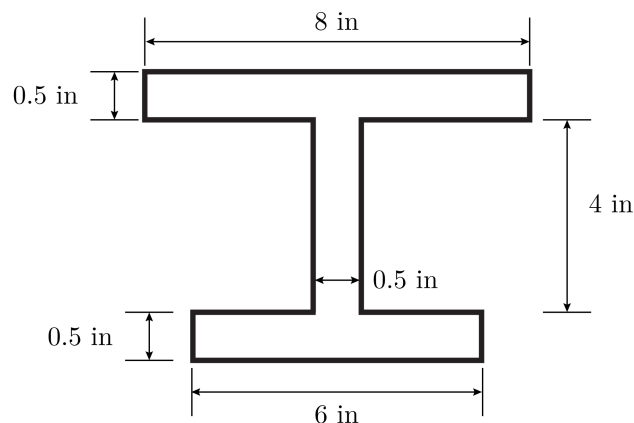


2. Given the cross-section of a beam below:

- a. (5 pts) Find the location of the Neutral Axis (N.A.) and moment of inertia about the N.A.

Given the max/min shear force ($V_{\max} = 4000$ lbs, $V_{\min} = -3200$ lbs) and bending moments ($M_{\max} = 7467$ lbs·ft, $M_{\min} = -9600$ lbs·ft), calculate the:

- b. (5 pts) Maximum tensile and compressive bending stresses (σ)
 c. (5 pts) Maximum shear stress (τ)



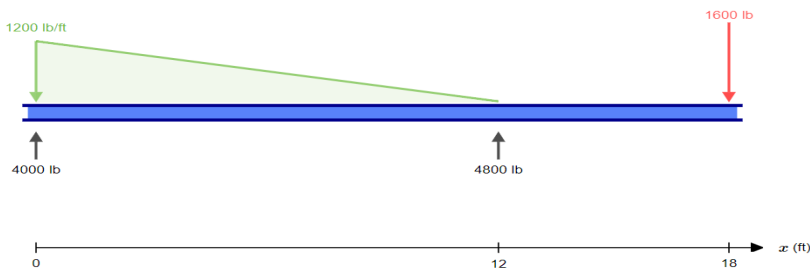
Problem 1

Executive Summary

Problem Statement

Draw V and M diagrams. Find maxima/minima and points of inflection.

Results



$$V_{max} = 4000 \text{ lbs}$$

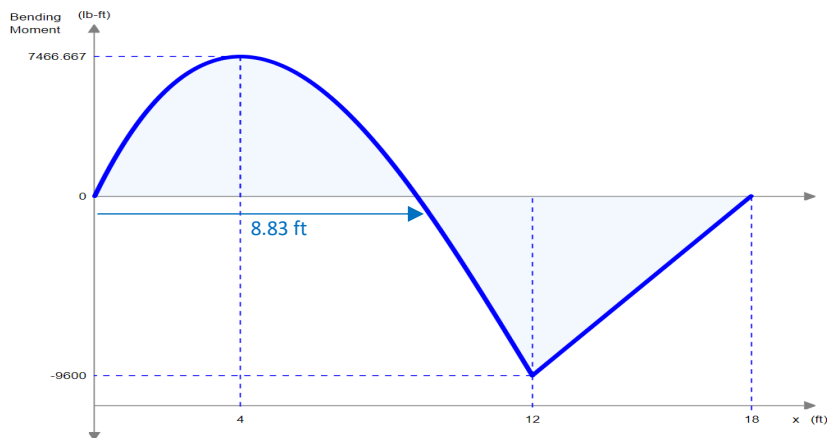
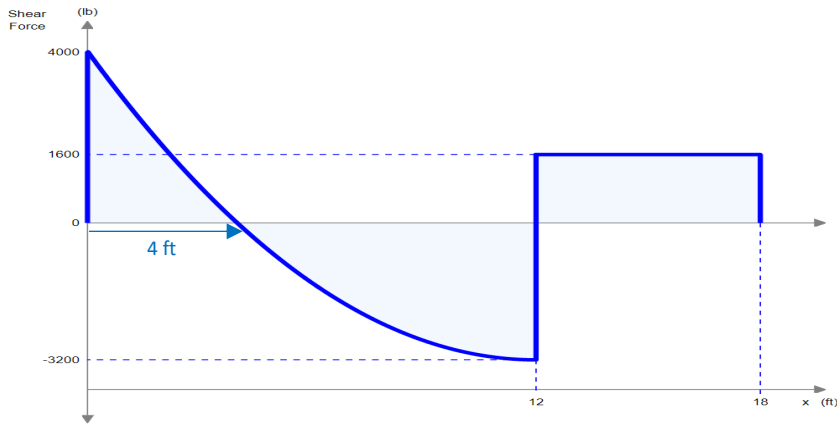
$$V_{min} = -3200 \text{ lbs}$$

$$M_{max} = 7466.7 \text{ lb} * \text{ft}$$

$$M_{min} = -9600 \text{ lb} * \text{ft}$$

Inflection point at

$$x = 8.83 \text{ ft}$$



Technical Summary

Step 1

Solve for resultant force and location.

$$F_R = \frac{1}{2} * 12ft * 1200 \frac{lb}{ft}$$
$$\Rightarrow F_R = 7200 \text{ lbs}$$

Load is triangular. F_R located at one – third distance away from left support.

$$\Rightarrow F_R @ x = 4ft$$

Step 2

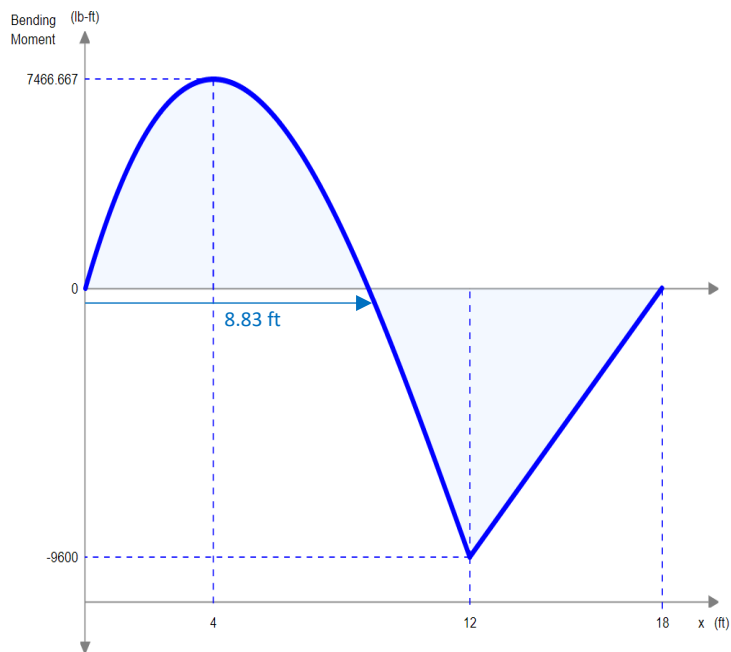
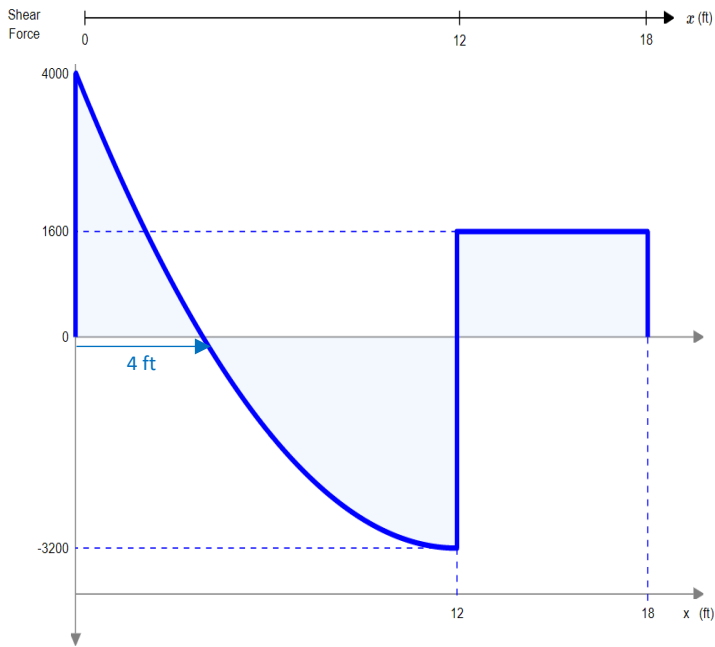
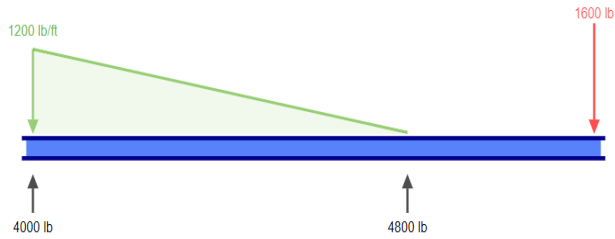
Solve for reactions at supports.

$$\Sigma M_A = 0 \rightarrow -F_R(4) + B_y(12) - 1600(18) = 0$$
$$\Rightarrow B_y = 4800 \text{ lbs}$$

$$\Sigma M_B = 0 \rightarrow F_R(8) - A_y(12) - 1600(6) = 0$$
$$\Rightarrow A_y = 4000 \text{ lbs}$$

Step 3

Draw V and M. Use integration to calculate V and M values.



$$q(x)_{0ft \rightarrow 12ft} = 100x - 1200$$

$$V(x) = \int q(x) dx$$

$$V(x) = 50x^2 - 1200x + C$$

$$V(0) = 4000 \rightarrow C = 4000$$

$$V(x)_{0ft \rightarrow 12ft} = 50x^2 - 1200x + 4000$$

$$\text{Solve: } V(x)_{0ft \rightarrow 12ft} = 0 \rightarrow x = 4ft$$

Moment curve reaches max at 4ft

$$V(0)_{0ft \rightarrow 12ft} = 4000 \text{ lbs}$$

$$V(4)_{0ft \rightarrow 12ft} = 0 \text{ lbs}$$

$$V(12)_{0ft \rightarrow 12ft} = -3200 \text{ lbs}$$

$$V(x)_{12ft \rightarrow 18ft} = -3200 + 4800 = 1600 \text{ lbs}$$

$$M(x) = \int V(x) dx$$

$$M(x) = \frac{50}{3}x^3 - 600x^2 + 4000x + D$$

$$M(0) = 0 \rightarrow D = 0$$

$$M(x)_{0ft \rightarrow 12ft} = \frac{50}{3}x^3 - 600x^2 + 4000x$$

$$\text{Solve: } M(x)_{0ft \rightarrow 12ft} = 0 \rightarrow x = 8.83ft$$

Inflection point at $x = 8.83ft$

$$M(0)_{0ft \rightarrow 12ft} = 0 \text{ lb} * \text{ft}$$

$$M(4)_{0ft \rightarrow 12ft} = 7466.7 \text{ lb} * \text{ft}$$

$$M(8.67)_{0ft \rightarrow 12ft} = 0 \text{ lb} * \text{ft}$$

$$M(12)_{0ft \rightarrow 12ft} = -9600 \text{ lb} * \text{ft}$$

$$M(18)_{12ft \rightarrow 18ft} = 0 \text{ lb} * \text{ft}$$

Step 4

Maxima and minima

$$V_{max} = 4000 \text{ lbs}$$

$$V_{min} = -3200 \text{ lbs}$$

$$M_{max} = 7466.7 \text{ lb} * \text{ft}$$

$$M_{min} = -9600 \text{ lb} * \text{ft}$$

Step 5

Inflection points

Inflection point at $M(x) = 0$

Inflection point at $x = 8.83 \text{ ft}$

Problem 2

Executive Summary

Problem Statement

Locate N.A.

Calculate max bending and shear stresses.

Results

$$\bar{y} = 2.75 \text{ in.}$$

$$I = 37.6874 \text{ in}^4$$

$$\sigma_{max}(T) = 6880 \text{ psi}$$

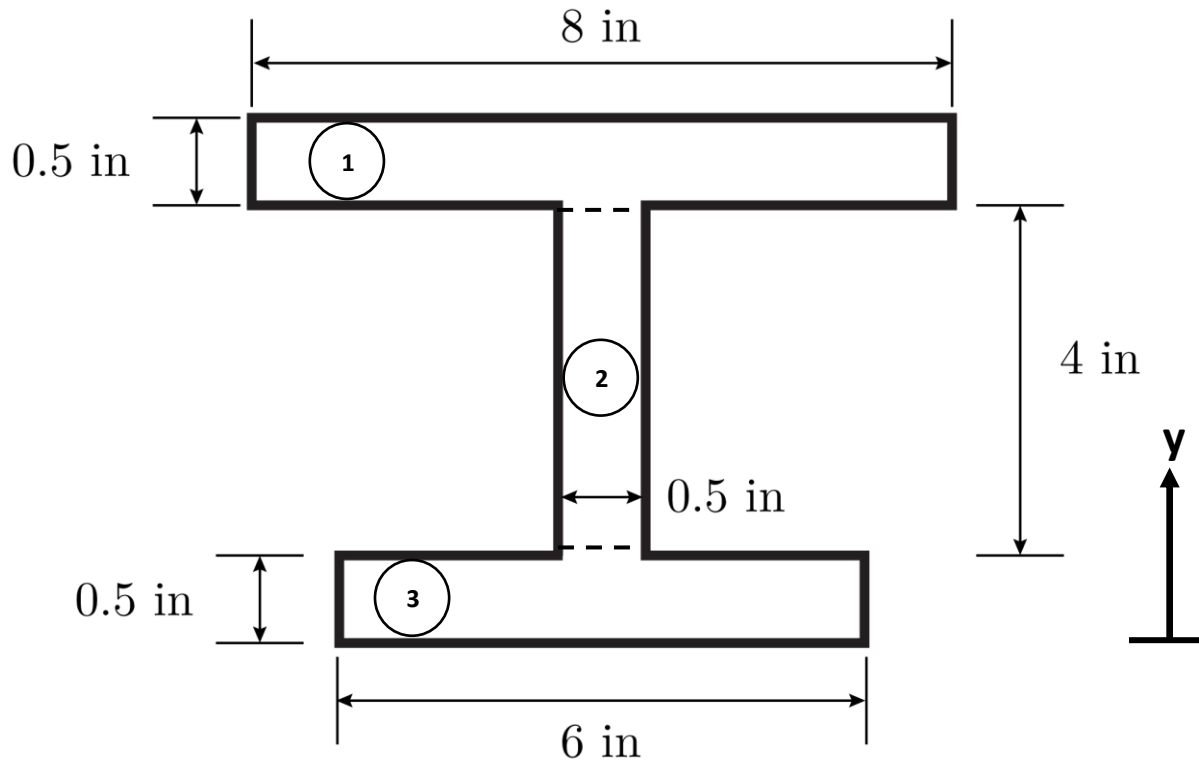
$$\sigma_{max}(C) = 8410 \text{ psi}$$

$$\tau_{xy_max} = 1860 \text{ psi}$$

Technical Summary

Step 1

Locate N.A.



$$\bar{y} = \frac{\sum_i \bar{y}_i \bar{A}_i}{\sum_i \bar{A}_i}$$

$$\bar{y} = \frac{((4.75)(4) + (2.5)(2) + (0.25)(3))}{(4 + 2 + 3)}$$

$$\bar{y} = 2.75 \text{ in.}$$

Step 2

Calculate Moment of Inertia

Section #	$I_{centroid}$	Area	$(\bar{y}_i - \bar{y})^2$	I_i
1	$\left(\frac{1}{12}\right)(8)(0.5)^3$	4	$(4.75 - 2.75)^2$	16.0833
2	$\left(\frac{1}{12}\right)(0.5)(4)^3$	2	$(2.5 - 2.75)^2$	2.7917
3	$\left(\frac{1}{12}\right)(6)(0.5)^3$	3	$(0.25 - 2.75)^2$	18.8125
			$I =$	37.6874 in^4

Step 3

Calculate Max Shear Stress

$$Q = \int y_i dA$$

$$Q(y = 0) = (4)(4.75 - 2.75) + (1.75 * 0.5) \left(2.75 + \frac{1.75}{2} - 2.75 \right)$$

$$Q(y = 0) = 8.7656 \text{ in}^3$$

Note: shear stress is maximum at the N.A.

Note: y measured from N.A.

$$V_{max} = 4000 \text{ lbs}$$

$$V_{min} = -3200 \text{ lbs}$$

$$\tau_{xy} = \frac{VQ}{Ib}$$

$$\tau_{xy_max} = \frac{(4000)(8.7656)}{(37.6874)(0.5)}$$

$$\tau_{xy_max} = 1860 \text{ psi}$$

Step 4

Calculate Max Bending Stress

$$M_{max} = 7466.7 \text{ lb} \cdot \text{ft}$$

$$M_{min} = -9600 \text{ lb} \cdot \text{ft}$$

$$\sigma_x = -\frac{My}{I}$$

Note: normal bending stress is maximum at extreme fibers

Note: y measured from N.A.

$$-\frac{(7466.7)(+2.25)(12)}{(37.6874)} = -5350 \text{ psi}$$

$$-\frac{(7466.7)(-2.75)(12)}{(37.6874)} = 6540 \text{ psi}$$

$$-\frac{(-9600)(+2.25)(12)}{(37.6874)} = 6880 \text{ psi}$$

$$-\frac{(-9600)(-2.75)(12)}{(37.6874)} = -8410 \text{ psi}$$

$$\sigma_{max}(T) = 6880 \text{ psi}$$

$$\sigma_{max}(C) = 8410 \text{ psi}$$